



Hydrodynamic Model Description

Isle of Rum, CAR/L/1152362

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1. Model Description

The hydrodynamic model used in the Rum Azamethiphos Dispersion Modelling (Mowi Scotland Ltd., 2022) was RiCOM (River and Coastal Ocean Model), a general-purpose hydrodynamics and transport model, which solves the standard Reynolds-averaged Navier-Stokes equation (RANS) and the incompressibility condition, applying the hydrostatic and Boussinesq approximations. It has been tested on a variety of benchmarks against both analytical and experimental data sets (e.g. Walters & Casulli 1998; Walters 2005a, b). The model has been previously used to investigate the inundation risk from tsunamis and storm surge on the New Zealand coastline (Walters 2005a; Gillibrand et al. 2011; Lane et al. 2011), to study tidal currents in high energy tidal environments (Walters et al. 2010) and, more recently, to study tidal energy resource (Plew & Stevens 2013; Walters et al. 2013; Walters 2016) and the effects of energy extraction on the ambient environment (McIlvenny et al. 2016; Gillibrand et al. 2016).

The basic equations considered here are the three-dimensional (3D) shallow water equations, derived from the Reynolds-averaged Navier-Stokes equations by using the hydrostatic assumption and the Boussinesq approximation. The continuity equation for incompressible flows is:

$$\nabla \cdot \mathbf{u} + \frac{\partial w}{\partial z} = 0 \quad (1)$$

where $\mathbf{u}(x,y,z,t)$ is the horizontal velocity vector, $w(x,y,z,t)$ is the vertical velocity, ∇ is the horizontal gradient operator, and z is the vertical coordinate. The momentum equation in non-conservative form is given by:

$$\frac{D\mathbf{u}}{Dt} + f\hat{\mathbf{z}} \times \mathbf{u} + \frac{1}{\rho_0} \nabla p - \frac{\partial}{\partial z} \left(A_V \frac{\partial \mathbf{u}}{\partial z} \right) - \nabla \cdot (A_h \nabla \mathbf{u}) + \mathbf{F} = 0 \quad (2)$$

where t is time; $f(x,y)$ is the Coriolis parameter; $\hat{\mathbf{z}}$ is the upward unit vector; $p(x,y,z,t)$ is pressure; ρ_0 is a reference density; $A_V(x,y,z,t)$ and $A_h(x,y,z,t)$ are the vertical and horizontal eddy viscosities respectively; \mathbf{F} represents body forces including form drag from obstacles in the flow; and x, y are the horizontal coordinates aligned to the east and north respectively.

The free surface equation is formed by vertically integrating the continuity equation and applying the kinematic free surface and bottom boundary conditions:

$$\frac{\partial \eta}{\partial t} - \nabla \cdot \left(\int_{-H}^{\eta} \mathbf{u} dz \right) = 0 \quad (3)$$

where H is the water depth relative to the mean level of the sea.

The model can be run in two- or three-dimensional mode. Frictional stress, $\boldsymbol{\tau}_b$, is applied at the seabed as a quadratic function of velocity:

$$\boldsymbol{\tau}_b = \rho C_D U_b |U_b| \quad (4)$$

where $\rho = 1025 \text{ kg m}^{-3}$ is the water density. The velocity, U_b , is either the velocity at the lowest sigma layer if the model is run in 3D or the depth-averaged velocity if run in 2D. The drag coefficient, C_D , can be either a constant or calculated from the bed roughness lengthscale, z_0 , using:

$$C_D = \left(\frac{\kappa}{\ln((z_b + z_0)/z_0)} \right)^2 \quad (5)$$

where $\kappa=0.4$ is von Karman's constant, and z_b is the height above the bed of the lowest velocity point.

Wind forcing was applied as a surface stress calculated from hourly wind speed and direction. Wind stress was calculated from the wind velocity by a standard quadratic relation:

$$\tau_x = \rho_a C_S u W \quad (6a)$$

$$\tau_y = \rho_a C_S v W \quad (6b)$$

where (u,v) are the East and North components of wind velocity respectively, W is the wind speed ($W = [u^2 + v^2]^{1/2}$), ρ_a is the density of air, and the surface drag coefficient C_S is calculated following Wu (1982) or Large and Pond (1981).

The equations are discretized on an unstructured grid of triangular elements which permits greater resolution of complex coastlines. The momentum and free surface equations are solved using semi-implicit techniques to optimize solution time and avoid the CFL stability constraint (Walters 2016). The material derivative in (2) is discretized using semi-Lagrangian methods to remove stability constraints on advection (Casulli, 1987; Walters et al. 2008). The Coriolis term is solved using a 3rd order Adams-Bashforth method (Walters et al. 2009). Full details of the model discretization and solution methods can be found in Walters et al. (2013) and Walters (2016). The solution methods provide a fast, accurate and robust code that runs efficiently on multi-core desktop workstations with shared memory using OpenMP. Full details of the model discretization and solution methods, including the basis of the application to tidal energy, are given by Walters et al. (2013) and Walters (2016).

2. Configuration and Boundary Forcing for Rum

The unstructured mesh used in the modelling (Figure 1) was adapted from the East Coast of Lewis and Harris (ECLH) sub-model mesh of the Scottish Shelf Model (SSM; MS, 2016). The model resolution was enhanced in the Small Isles region, particularly around the Mowi site at Rum (Figure 2). The spatial resolution of the model varied from 21m in some inshore waters and round the farm pens to 5km along the open boundary. The model consisted of 75,331 nodes and 142,263 triangular elements. The model was run in 2D mode.

The model is forced at the outer boundaries by 8 tidal constituents (M_2 , S_2 , N_2 , K_2 , O_1 , K_1 , P_1 and Q_1) which were derived from tidal analysis (Pawlowicz et al., 2002) of the sea surface elevations at the closest nodes from the Scottish Shelf Model climatology (Marine Scotland, 2016). Spatially- and temporally-varying wind speed and direction data are taken from the ERA5 global reanalysis dataset (ECMWF, 2021) for the required simulation periods.

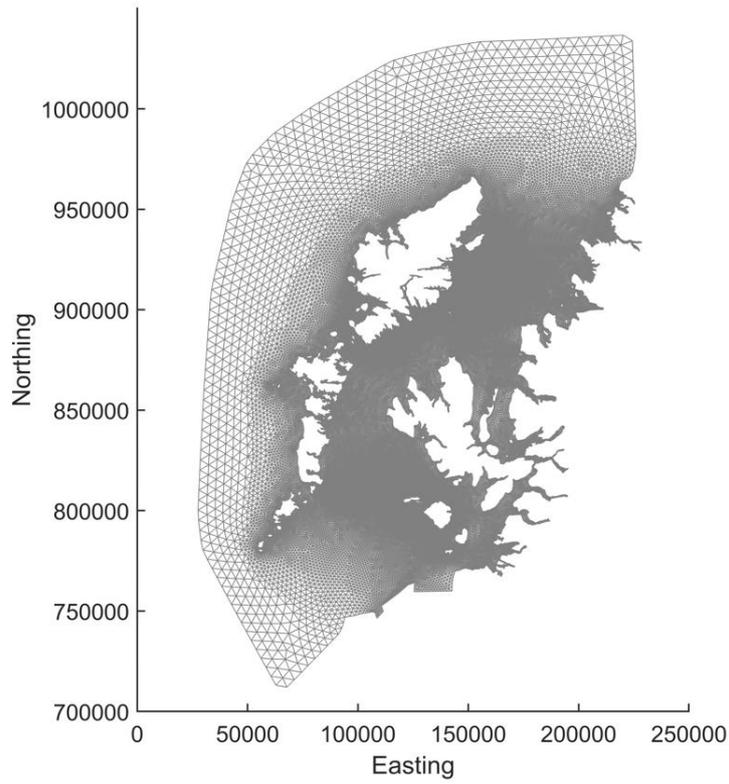


Figure 1. The ECLH mesh and domain of the modelling study (SSM)

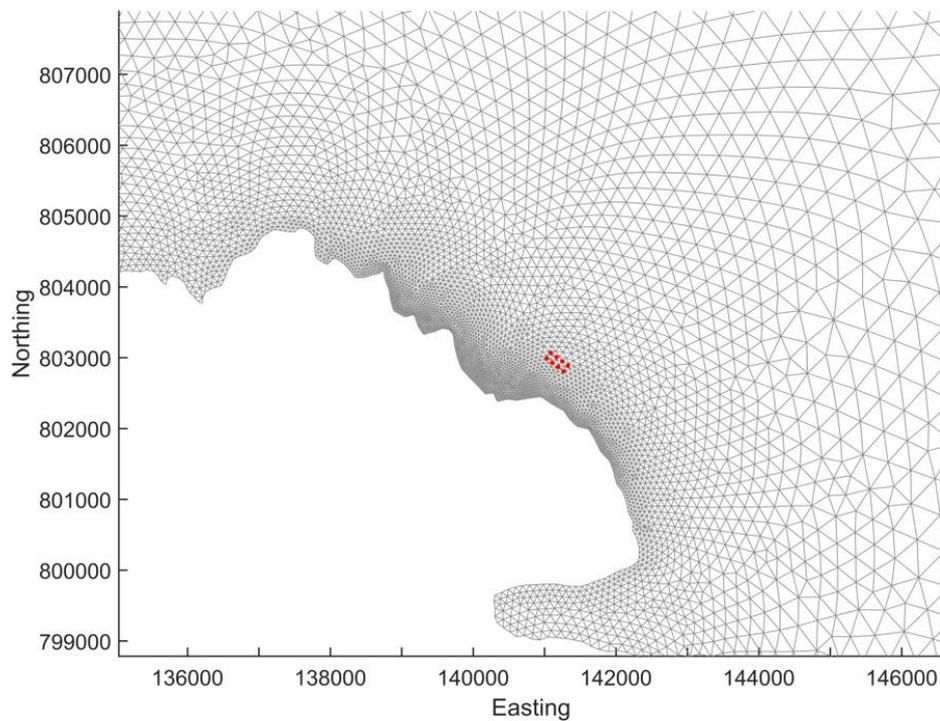


Figure 2. The unstructured mesh around the Rum site in the modified model grid, with the proposed cage locations indicated (•).

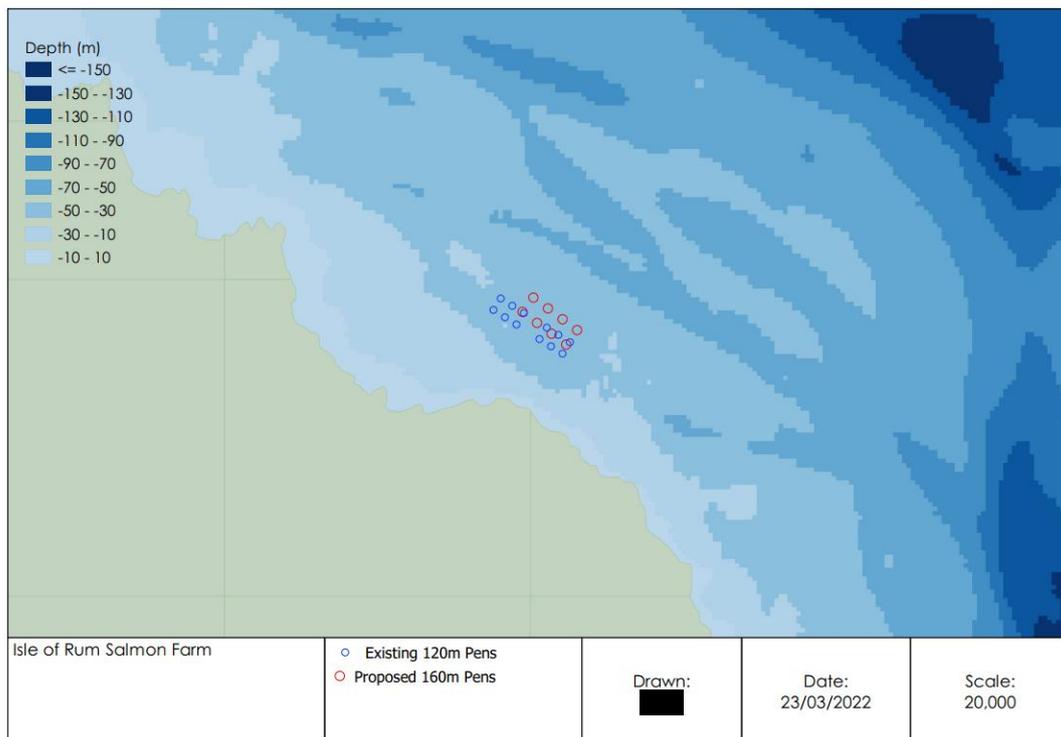


Figure 3. Model water depths (m) in the area around Rum salmon farm.

3. Model Calibration and Validation

The RiCOM model has previously been calibrated against sea level and current meter data from the north of Scotland (Gillibrand et al. 2016). For the current study, the model was further calibrated against hydrographic data collected in the region of the farm site in 2016 and again in 2018. The data are described in the relevant hydrographic reports. In August 2016, an Acoustic Doppler Current Profiler (ADCP) was deployed close to the farm site (Figure 4) until October 2016 (ID113). In August 2018, another ADCP was deployed close to the farm site (Figure 4) until December 2018 (ID242). In all, 139 days of current data were used in this application. ADCP deployments provided both current velocity and seabed pressure data, which were used to calibrate and validate modelled velocity and sea surface height. The model was calibrated initially against data from August – October 2016 (ID113), then validated against data from the other deployment, ID242 from 2018.

For each simulation, the model was “spun-up” for three days with boundary forcing ramped up from zero over a period of 48 hours. The model state at the end of the 72-hour spin-up period was stored, and the main simulations “hot-started” from this state.

The following main simulations were performed, corresponding with the dates of the ADCP deployments:

- (i) Calibration: 15th August 2016 – 9th October 2016 (ID113)
- (ii) Validation: 30th August 2018 – 21st December 2018 (ID242)

[Note that the dates above refer to the main simulations and that the spin-up simulations ran for three days prior to the start dates given above.]

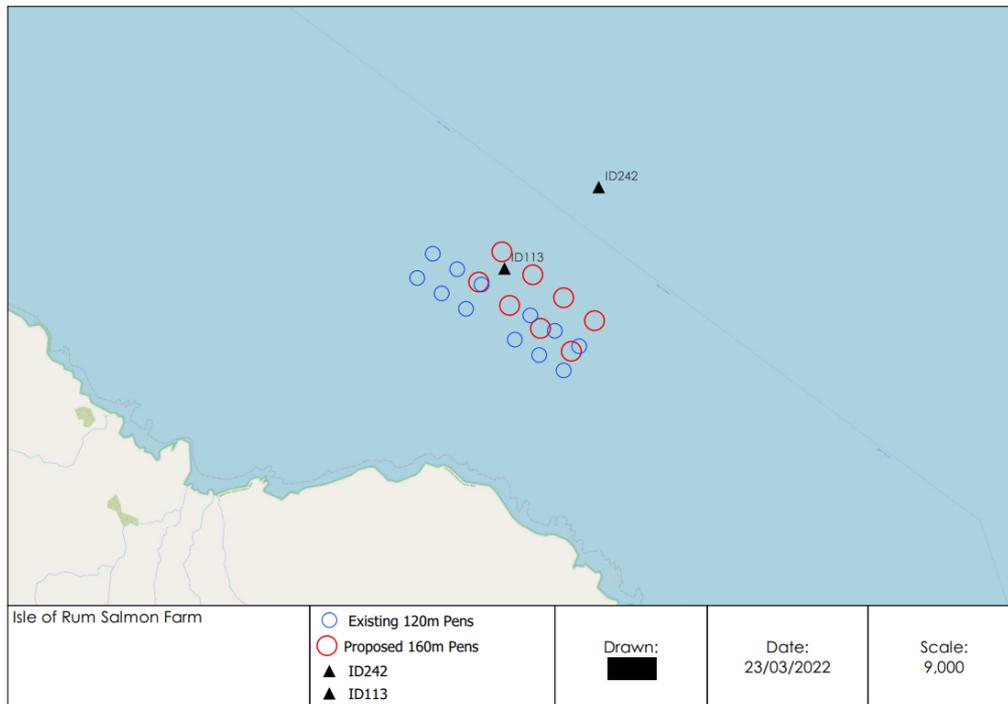


Figure 4. Locations of the ADCP deployments relative to the proposed 8 pens at the Isle of Rum site.

Model performance is assessed using three metrics: the mean absolute error (MAE), the root-mean-square error (RMSE) and the model skill (d_2). The first two are standard measures of model accuracy; the third, d_2 , is taken from Willmott et al. (1985) and lies in the range $0 \leq d_2 \leq 1$, with $d_2 = 0$ implying zero model skill and $d_2 = 1$ indicating perfect skill.

3.1 Calibration: August – October 2016, ID113

The calibration used observed depth and current velocity from the ADCP location to compare with modelled sea surface height (SSH) and velocity (ADCP deployment ID113). The model was calibrated by varying the value of the drag coefficient, C_D , in Equation A4, which determines the frictional effect of the seabed on the flow. Simulations were performed with a range of values of C_D , varying over the range $0.002 \leq C_D \leq 0.02$. After a number of simulations, a final parameter set was selected (Table 1).

Table 1. Parameter values chosen for the RiCOM model during the calibration simulations.

Parameter Description	Value
Drag coefficient, C_D	0.013
Number of vertical levels	1
Model time step (s)	72

The results of the calibration exercise are presented in Figure 5 – Figure 8 and Table 2. At the ADCP location, the sea surface height was reasonably accurately modelled, with model skill of 0.99. The mean absolute error (MAE) and root-mean-square error (RMSE) values of 0.14 m and 0.17 are about 2.8 % and 3.4 % of the spring tide range (~5m) respectively.

For the calibration period, the model skill scores were 0.84 and 0.84 for the East and North components of velocity respectively. RMSE values were both 0.06 (Table 2). The scatter plots and histograms demonstrate that the modelled current had broadly the same magnitude and direction characteristics as the observed data (Figure 7 and Figure 8).

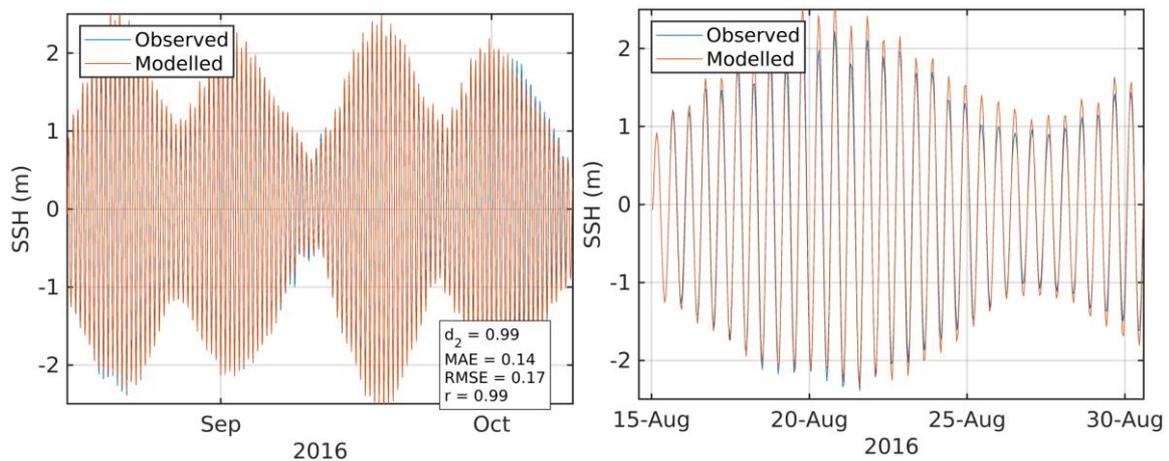


Figure 5. Comparison between observed and modelled sea surface height from August – October 2016 (ADCP deployment ID113) using model parameter values from Table 1. Both the full record (left) and a subset of 15 days (right) are shown. Observed data are in blue, model results in red.

Table 2. Model performance statistics for sea surface height (SSH) and East and North velocity at the ADCP location from August – October 2016 (ID113).

	SSH	East	North
Model skill, d_2	0.99	0.84	0.84
Mean Absolute Error (MAE)	0.14 m	0.05 m s ⁻¹	0.05 m s ⁻¹
Root-Mean-Square Error (RMSE)	0.17 m	0.06 m s ⁻¹	0.06 m s ⁻¹

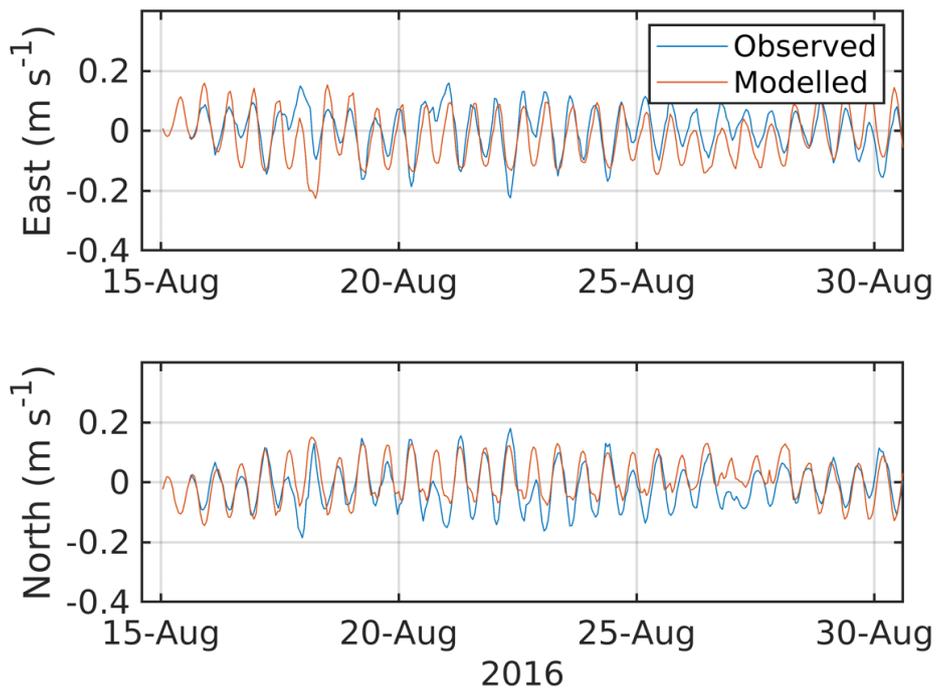


Figure 6. Comparison between observed and modelled East (top) and North (bottom) components of velocity at the ADCP location for 15 days in August - October 2016 (ID113). Observed data are in blue, model results in red.

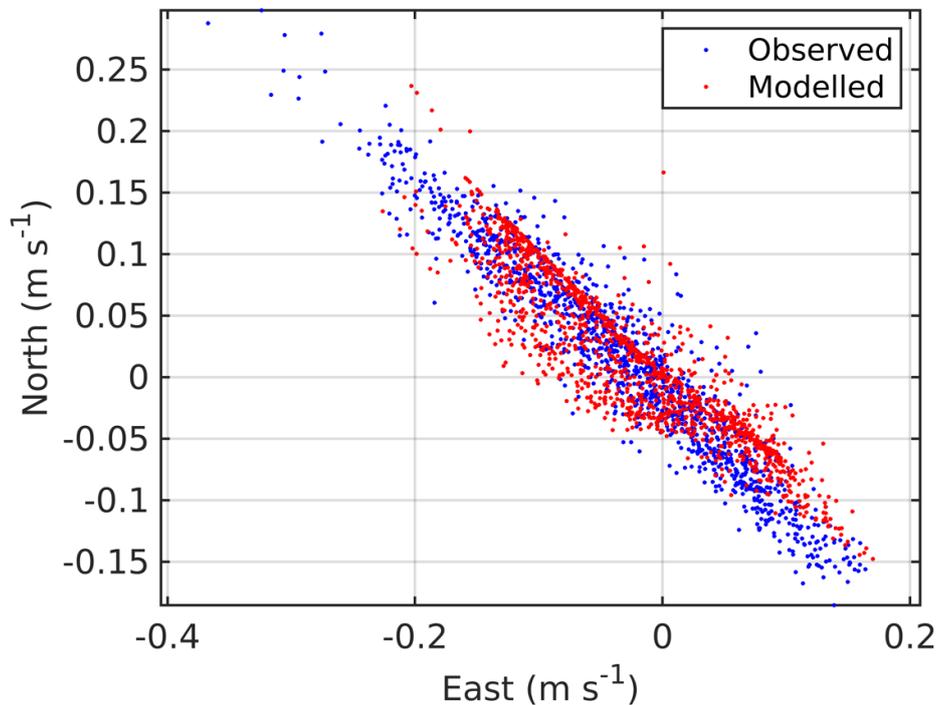


Figure 7. Scatter plot of observed and modelled velocity at the ADCP location from August - October 2016 (ID113). Observed data are in blue, model results in red.

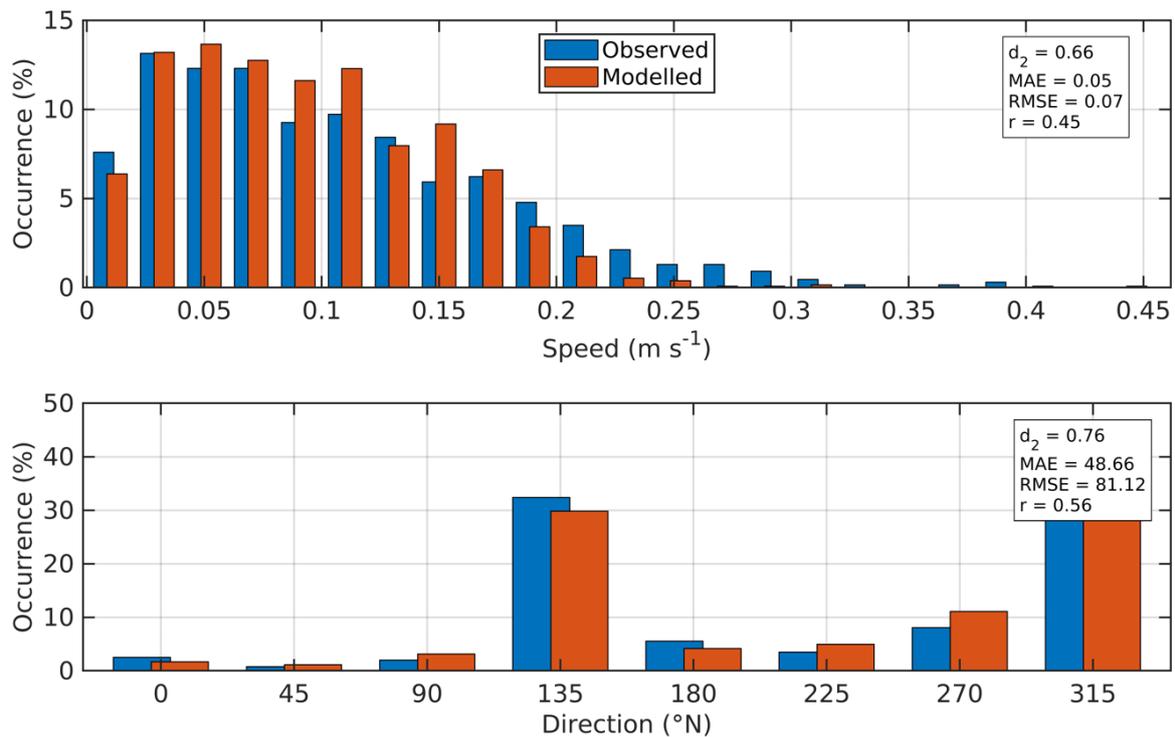


Figure 8. Histograms of observed and modelled speed (top) and direction (bottom) at the ADCP location from August – October 2016 (ID113). Observed data are in blue, model results in red.

3.2 Validation: August – December 2018, ID242

At the ADCP location, the sea surface height was reasonably accurately modelled, with model skill of 0.99 (Figure 9, Table 3). The mean absolute error (MAE) and root-mean-square error (RMSE) values of 0.18 m and 0.21 m are about 3.6 % and 4.2 % of the spring tide range (~5m) respectively.

North and east components of velocity at the ADCP location were satisfactorily reproduced by the model, with values of the model skill, d_2 , of 0.79 and 0.76 respectively (Figure 10, Table 3). The values of the MAE and RMSE being in the range 5 – 7 cm s⁻¹ (Table 3). The scatter plots and histograms shown in Figure 11 and Figure 12 demonstrate that the modelled currents were broadly of the same speed and direction as the observed data.

Table 3. Model performance statistics for sea surface height (SSH) and East and North velocity at the ADCP location from August – December 2018 (ID242).

	SSH	East	North
Skill, d_2	0.99	0.79	0.76
Mean Absolute Error (MAE)	0.18 m	0.06 m s ⁻¹	0.05 m s ⁻¹
Root-Mean-Square Error (RMSE)	0.21 m	0.07 m s ⁻¹	0.07 m s ⁻¹

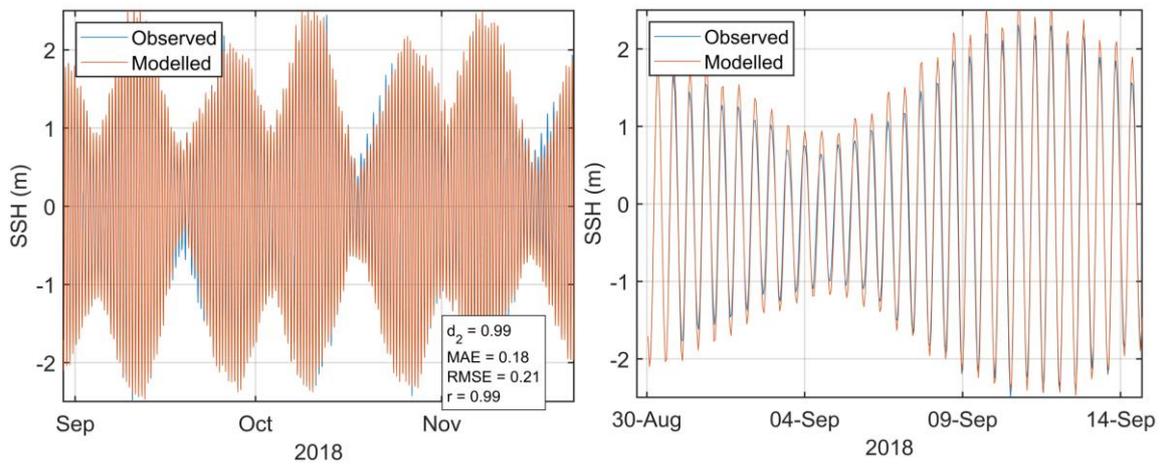


Figure 9. Comparison between observed and modelled sea surface height from August – December 2018 (ADCP deployment ID242) using model parameter values from Table 1. Both the full record (left) and a subset of 15 days (right) are shown. Observed data are in blue, model results in red.

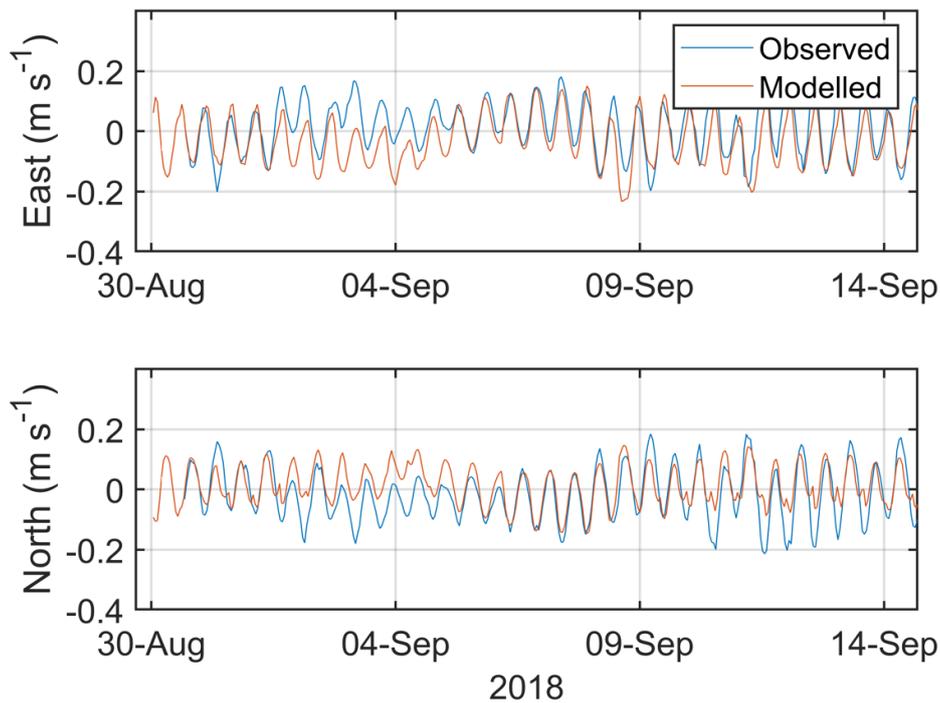


Figure 10. Comparison between observed and modelled East (top) and North (bottom) components of velocity at the ADCP location for 15 days in August – December 2018 (ID242). Observed data are in blue, model results in red.

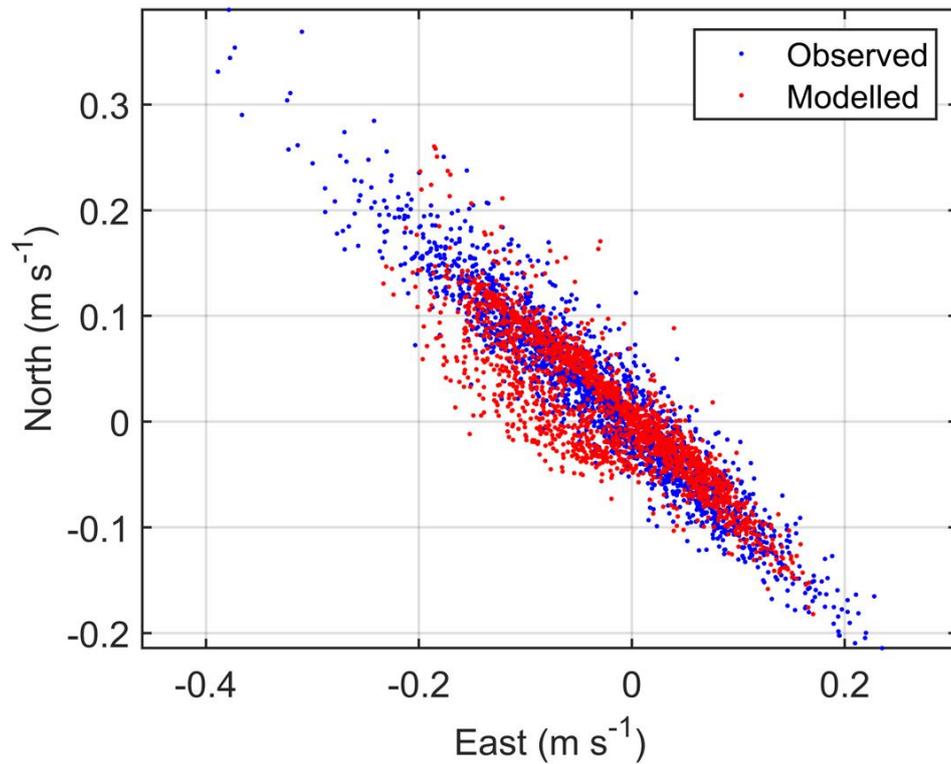


Figure 11. Scatter plot of observed and modelled velocity at the ADCP location from August – December 2018 (ID242). Observed data are in blue, model results in red.

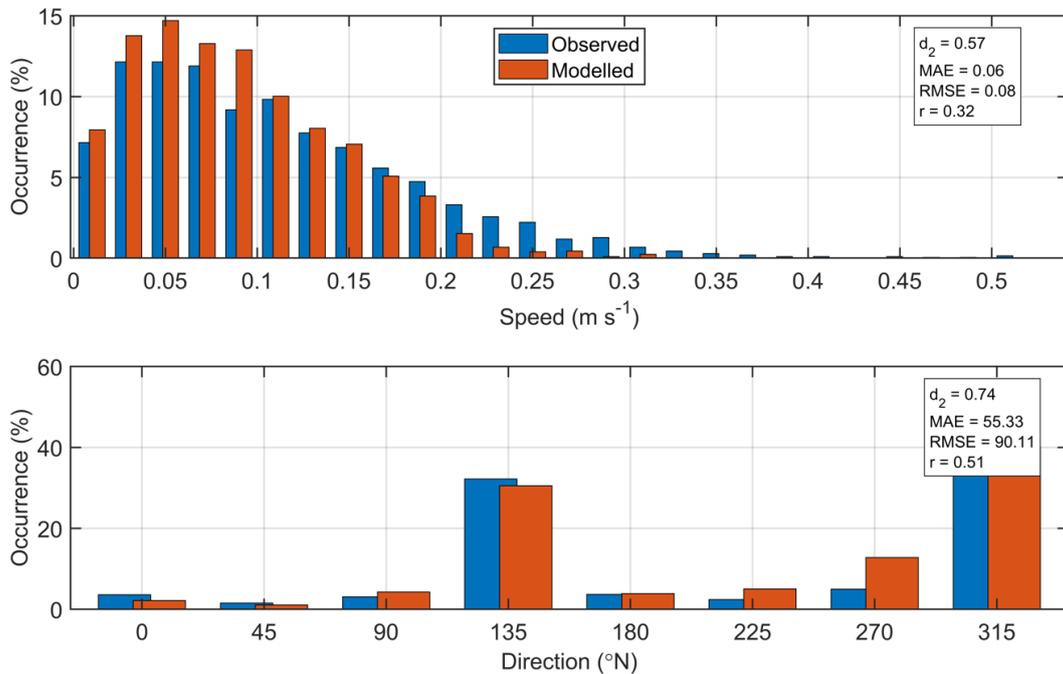


Figure 12. Histograms of observed and modelled current speed (top) and direction (bottom) at the ADCP location from August – December 2018 (ID242). Observed data are in blue, model results in red.

4. Modelled Flow Fields

Modelled flood and ebb velocity vectors at spring tides are illustrated in Figure 13. The Rum site is exposed to the strong currents from Cuillin Sound. The prevailing flow is north-westwards which is typical of the Scottish Coastal Current. Currents are strong around the Rum site, with a mean near-surface speed exceeding 0.13 m s^{-1} .

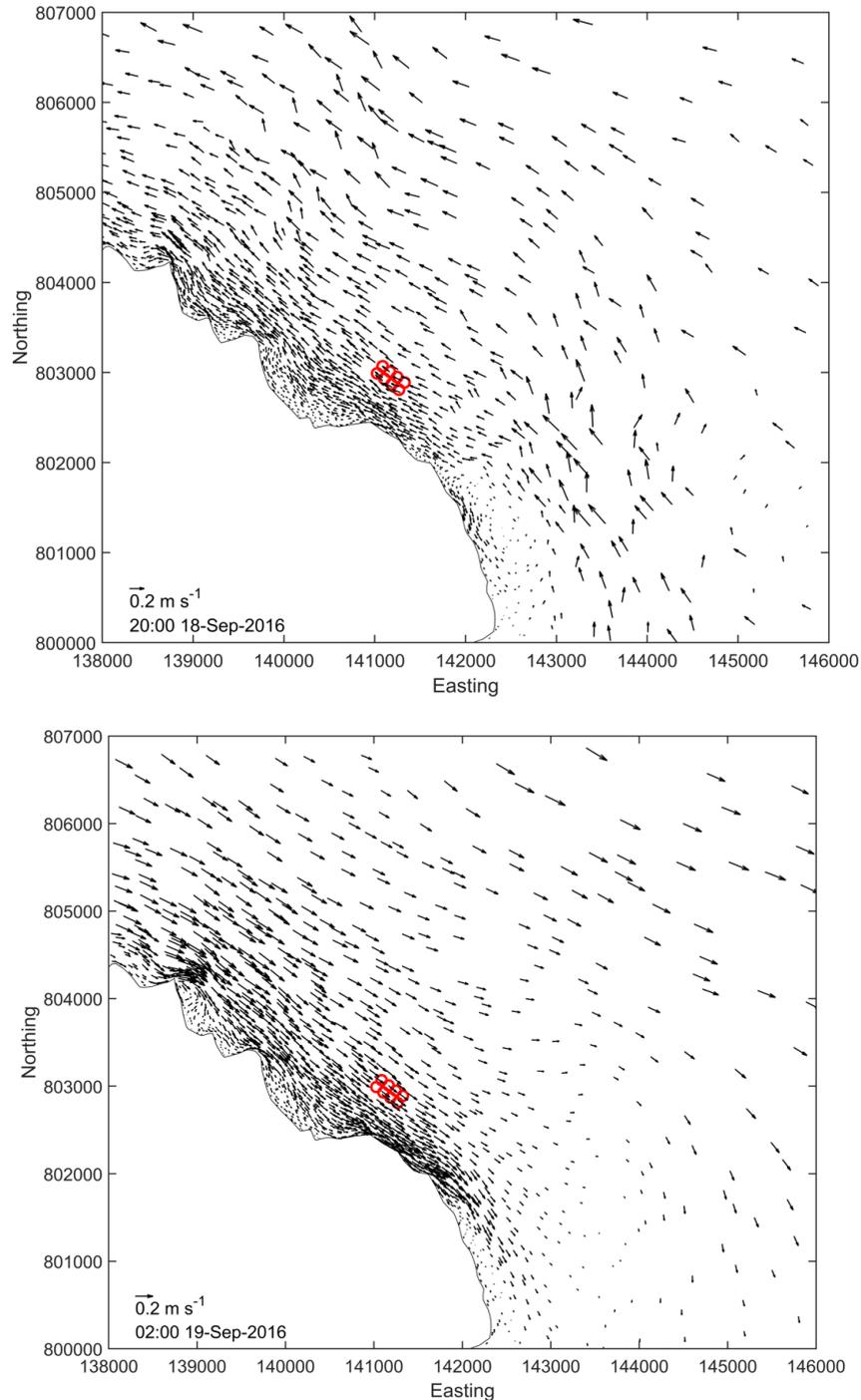


Figure 13. Modelled flood (top) and ebb (bottom) surface current vectors during spring tides. For clarity, only every 5th element of the model vectors are shown.

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